

GENERAL

- 1. STRUCTURAL DIMENSIONS CONTROLLED BY OR RELATED TO MECHANICAL OR ELECTRICAL EQUIPMENT SHALL BE VERIFIED BY THE CONTRACTOR PRIOR TO CONSTRUCTION.
2. MECHANICAL AND ELECTRICAL EQUIPMENT SUPPORTS, ANCHORAGES, OPENINGS, RECESSES AND REVEALS NOT SHOWN ON THE STRUCTURAL DRAWINGS BUT REQUIRED BY OTHER CONTRACT DRAWINGS, SHALL BE PROVIDED FOR PRIOR TO PLACING CONCRETE.
3. STRUCTURAL DRAWINGS SHALL BE USED IN COORDINATION WITH MECHANICAL, ELECTRICAL, ARCHITECTURAL, CIVIL DRAWINGS AND SHOP DRAWINGS PROVIDED BY MANUFACTURERS OF EQUIPMENT.
4. STRUCTURES HAVE BEEN DESIGNED FOR OPERATIONAL LOADS ON THE COMPLETED STRUCTURES. DURING CONSTRUCTION, THE STRUCTURES SHALL BE PROTECTED BY BRACING AND BALANCING WHEREVER EXCESSIVE CONSTRUCTION LOADS MAY OCCUR.
5. UNLESS OTHERWISE SHOWN, ON ALL STRUCTURAL DRAWINGS THE FINISH GRADE AROUND STRUCTURES IS SHOWN THUS INDICATING EITHER GROUND SURFACE, TOP OF CONCRETE SLAB OR A.C. PAVEMENT. FOR DETAILS OF FINISH SURFACES SEE CIVIL AND ARCHITECTURAL DRAWINGS.

- 6. LOCATION OF ALL CONSTRUCTION JOINTS SHALL BE AS SHOWN ON THE DRAWINGS OR APPROVED BY THE ENGINEER. ALL CONSTRUCTION JOINTS LOCATED ON THE DRAWINGS OR REQUIRED FOR CONSTRUCTION BUT NOT SHOWN ON THE DRAWINGS, SHALL HAVE A WATERSTOP APPROVED BY THE ENGINEER IF MEMBERS IN CONTACT WITH WATER.
7. UNLESS OTHERWISE NOTED, CONSTRUCTION JOINTS OF ALL WATER BEARING STRUCTURES SHALL HAVE A 6" PVC WATERSTOP AND A SEALANT GROOVE.

STRUCTURAL LOADING

- 1. DESIGN IN ACCORDANCE WITH THE 2006 INTERNATIONAL BUILDING CODE WITH 2009 GEORGIA STATE AMENDMENTS TO IBC 2006 AND ASCE/SEI 7-05, MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES. EXCEPT WHERE OTHER APPLICABLE CODES OR THE FOLLOWING NOTES ARE MORE RESTRICTIVE.

DEAD LOADS

THE WEIGHT OF MEMBER ITSELF AND ALL PERMANENT CONSTRUCTION INCLUDING WALLS, FLOORS, ROOFS, CEILINGS AND FIXED SERVICE EQUIPMENT SHALL BE CONSIDERED TO BE PART OF THE DEAD LOADS. AN ALLOWANCE FOR ROUTED MECHANICAL, PIPING, HVAC AND ELECTRICAL EQUIPMENT OF 15 PSF SHALL ALSO BE INCLUDED ON THE FLOORS AND ROOFS EXCEPT ON MODULAR WOOD STRUCTURES.

LIVE LOADS

- MINIMUM UNIFORMLY DISTRIBUTED LIVE LOADS SHALL BE AS FOLLOWS:
ROOFS 20.0 PSF
PLATFORM, WORK AREA, FLOORS, CATWALKS, & PERSONNEL ASSEMBLY AREAS 100 PSF
LIGHT STORAGE 125 PSF
LABORATORIES 125 PSF
HEAVY STORAGE 250 PSF
MECHANICAL & ELECTRICAL ROOMS 150 PSF
STAIRS, FIRE ESCAPES, CORRIDORS, & EXIT WAYS 100 PSF UNIFORM AND 1,000 POUNDS CONCENTRATED
SLAB ON GRADE & VEHICULAR LOADS 250 PSF OR FORK LIFT TRUCK, 6 KIP CAPACITY (HS20-44 CAPACITY IN DESIGNATED AREAS)
PUMP STATION, PROCESS BUILDING, & PROCESS SLAB 250 PSF
PARTITIONS 20 PSF
SIDEWALKS & DRIVEWAYS 250 PSF
SLUICE GATE PER MANUFACTURE'S RECOMMENDATIONS (MIN 100 POUND ON HANDLE ALL DIRECTIONS)
GRATING, CHECKERED PLATES & HATCHES MINIMUM OF 100 PSF AND 300 POUNDS CONCENTRATED, BUT NOT LESS THAN LOADING FOR ADJACENT FLOOR AREA (BUT NOT LESS THAN 200 PSF FOR GRATING). THE MAXIMUM DEFLECTION UNDER THIS LOADING SHALL BE LIMITED TO L/240
BURIED STRUCTURE USE AASHTO STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES, 17TH EDITION, 2002. FOR STRUCTURES IN POTENTIAL TRAFFIC AREAS

- TRAVELLING BRIDGE CRANE AND HOIST:
DIVERSION PUMP STATION (EXTERIOR) 10-TON + 25% IMPACT (BRIDGE CRANE)
DIVERSION PUMP STATION (BASEMENT) 4-TON + 25% IMPACT (HOIST/CRANE)
JET MIX PUMP STATION (BASEMENT) 5-TON + 25% IMPACT (BRIDGE CRANE)

LIQUID LOADS

- LIQUID HOLDING BASIN WALLS SHALL BE DESIGNED FOR MAXIMUM LIQUID LEVELS WITH THE FOLLOWING CONDITIONS:
FULL OF LIQUID, NO BACKFILL.
BACKFILL AND GROUND WATER WITH TANK EMPTY.
ANY TANK CELL EMPTY OR FULL IN ANY COMBINATION.
STATIC EARTH PRESSURE PLUS SURCHARGE WITH TANK EMPTY.
DYNAMIC EARTH PRESSURE WITH TANK EMPTY.

STRUCTURAL LOADING CONT.

IMPACT LOAD
SHALL BE DEFINED AS AN EQUIVALENT STATIC FORCE CAUSED BY A MOVING OBJECT. DYNAMIC LOAD FACTORS (DLF) SHALL BE PER THE MINIMUM REQUIREMENT IN SUCH INDUSTRY STANDARDS AS AASHTO, AISC, ASCE 7, OR AS REQUIRED BY THE EQUIPMENT MANUFACTURER.

THERMAL LOAD
SHALL BE DEFINED AS THOSE FORCES CAUSED BY A CHANGE IN TEMPERATURE. SUCH FORCES SHALL INCLUDE BUT NOT BE LIMITED TO THOSE CAUSED BY PIPING EXPANSION OR CONTRACTION.

ERECTION LOAD
SHALL BE DEFINED AS TEMPORARY FORCES CAUSED BY ERECTION OF STRUCTURES OR EQUIPMENT.

EQUIPMENT LOADS
SHALL BE PROVIDED BY VENDORS AND SHALL INCLUDE TEST LOAD(S) WHEN LARGER THAN THE OPERATING LOAD(S) OF EQUIPMENT.

SNOW LOADS
GROUND SNOW LOADS 5 PSF

WIND LOADS
BASIC WIND SPEED 90 MPH WITH 3-SECOND GUST WIND.
GUST EFFECT FACTOR, G 0.85
EXPOSURE C
OCCUPANCY CATEGORY III
IMPORTANCE FACTOR, I 1.15

WIND DIRECTIONALITY (SHAPE) FACTOR, KD: PER TABLE 6-4, ASCE/SEI 7-05

SEISMIC LOADS
STATIC LATERAL FORCE PROCEDURE
OCCUPANCY CATEGORY III
SEISMIC DESIGN CATEGORY (SDC) B
IMPORTANCE FACTOR, I 1.25
SITE CLASSIFICATION D
SHORT-PERIOD SITE COEFFICIENT, FA 1.6
LONG-PERIOD SITE COEFFICIENT, FV 2.4
MAPPED MCE, 5% DAMPED, SPECTRAL RESPONSE ACCELERATION PARAMETER AT SHORT PERIODS, Ss 0.20
MAPPED MCE, 5% DAMPED, SPECTRAL RESPONSE ACCELERATION PARAMETER AT A PERIOD OF 1s, S1 0.08

THE "MCE" SPECTRAL RESPONSE ACCELERATION FOR SHORT PERIOD, Sms = FxSs = 0.32
THE "MCE" SPECTRAL RESPONSE ACCELERATION AT 1 SECOND, Sm1 = FvS1 = 0.19

DESIGN EARTHQUAKE SPECTRAL RESPONSE ACCELERATION PARAMETER AT SHORT PERIOD, Sds = (2/3)Sms = 0.213
DESIGN EARTHQUAKE SPECTRAL RESPONSE ACCELERATION PARAMETER AT 1s PERIOD, Sd1 = (2/3)Sm1 = 0.127

BUILDING STRUCTURES, SEISMIC FORCE-RESISTING SYSTEMS, RESPONSE MODIFICATION COEFFICIENT, R:
a. ORDINARY REINFORCED CONCRETE MOMENT FRAMES, R = 3
b. ORDINARY STEEL MOMENT FRAMES, R = 3.5

SEISMIC RESPONSE COEFFICIENT, Cs = (Sds/(R/I)) BUT NOT LESS THAN 0.01:
a. FOR ORDINARY REINFORCED CONCRETE MOMENT FRAMES, Cs = 0.088
b. FOR ORDINARY STEEL MOMENT FRAMES, Cs = 0.075

BASE SHEAR, V = CsW, WHERE W = THE EFFECTIVE SEISMIC WEIGHT

SEE CIVIL DRAWINGS AND SPECIFICATIONS SECTION 2200 FOR EARTHWORK PREPARATION OF FOUNDATIONS INCLUDING THE REMOVAL OF ORGANIC MATERIALS, SCARIFYING, COMPACTING SOILS BENEATH STRUCTURES, BACKFILL REQUIREMENTS FOR OVER EXCAVATION AND REMOVAL OF UNSUITABLE MATERIALS BENEATH STRUCTURES. ALL MATERIALS UNDER FOUNDATION SHALL BE COMPACTED TO 95% LEVEL.

SEE SOILS REPORT FOR THE ADDITIONAL REQUIREMENTS OF BASE MATERIALS AND PREPARATIONS PRIOR TO THE PLACEMENT OF CONCRETE FOUNDATION.

DESIGN OF FOUNDATIONS ARE BASED ON THE RECOMMENDATIONS OF THE GEOTECHNICAL INVESTIGATION REPORT BY WILMER ENGINEERING INC., PROJECT #71.3801, DATED JUNE 14, 2012.

THIS REPORT IS MADE PART OF THE CONTRACT DOCUMENTS.

SEE CIVIL DRAWINGS AND SPECIFICATIONS SECTION 2200 FOR EARTHWORK PREPARATION OF FOUNDATIONS INCLUDING THE REMOVAL OF ORGANIC MATERIALS, SCARIFYING, COMPACTING SOILS BENEATH STRUCTURES, BACKFILL REQUIREMENTS FOR OVER EXCAVATION AND REMOVAL OF UNSUITABLE MATERIALS BENEATH STRUCTURES. ALL MATERIALS UNDER FOUNDATION SHALL BE COMPACTED TO 95% LEVEL.

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STRUCTURAL STEEL

- 1. ALL MISCELLANEOUS STRUCTURAL STEEL, ANGLES AND PLATES SHALL CONFORM TO ASTM A36. WIDE FLANGE SHAPES SHALL CONFORM TO ASTM A992.
2. ALL PIPES USED AS STRUCTURAL MEMBER SHALL BE ASTM A53, GRADE "B".
3. ALL TUBULAR STEEL SECTIONS SHALL CONFORM TO ASTM A500 GRADE "B" (FY = 48 KSI).
4. ROUND HOLLOW STRUCTURAL SECTIONS (HSS) APPROVED, USE HIGH-STRENGTH LOW ALLOY STEEL A500, GRADE B, fy=42ksi.
5. ALL FABRICATION AND ERECTION SHALL CONFORM "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS, ANSI/AISC 360-05 EDITION AND AISC STEEL CONSTRUCTION MANUAL, 13TH EDITION.
6. NO STRUCTURAL STEEL SHALL BE FABRICATED OR ERECTED UNTIL SHOP DRAWINGS HAVE BEEN REVIEWED BY THE STRUCTURAL ENGINEER OF RECORD.
7. ALL WELDING SHALL BE PERFORMED IN ACCORDANCE WITH AWS D1.1 BY CERTIFIED WELDERS.
8. NO FIELD CUTTING OR BURNING OF STRUCTURAL STEEL WILL BE PERMITTED WITHOUT WRITTEN APPROVAL OF THE STRUCTURAL ENGINEER OF RECORD.
9. BASE PLATES SHALL BE LEVELED WITH DOUBLE NUTS. LEVELING PLATES ALLOWED ONLY AT ENGINEERS APPROVAL.
10. BOLTS SHALL CONFORM TO ASTM A325-N, UNON.

CONCRETE (CAST-IN-PLACE)

- 1. CEMENT SHALL BE PORTLAND CEMENT CONFORMING TO ASTM C150, TYPE I/II. TYPE II CEMENT SHALL BE USED IN ALL WATER BEARING STRUCTURES.
2. CONCRETE CONSTRUCTION SHALL CONFORM TO ACI 318-05 AND ACI 350-01 AND COMMENTARY ACI 350R-01
3. UNLESS OTHERWISE NOTED OR SPECIFIED, ALL STRUCTURAL CONCRETE SHALL DEVELOP A MINIMUM COMPRESSIVE STRENGTH OF 4,000 PSI IN 28 DAYS.
4. REINFORCEMENT STEEL SHALL BE DEFORMED BARS CONFORMING IN QUALITY TO THE REQUIREMENTS OF THE "SPECIFICATIONS FOR DEFORMED CARBON STEEL BARS FOR CONCRETE REINFORCEMENT" ASTM DESIGNATION A615, GRADE 60, UN.
5. ALL DETAILING, FABRICATION AND PLACING OF REINFORCING BARS, UNLESS OTHERWISE NOTED, SHALL BE IN ACCORDANCE WITH THE "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES" ACI 315, LATEST EDITION.
6. TOLERANCES IN PLACING REINFORCEMENT SHALL BE:
± 3/8 INCH FOR MEMBERS WITH ≤ D 8 INCHES
± 1/2 INCH FOR MEMBERS WITH > D 8 INCHES
7. ALL KEYWAYS IN CONSTRUCTION JOINTS SHALL BE ROUGHENED WITH 1/4" AMPLITUDE AND THOROUGHLY CLEANED FOR BOND.
8. ALL REINFORCING STEEL, ANCHOR BOLTS, DOWELS AND OTHER INSERTS AND EMBEDS SHALL BE SECURED IN POSITION, INSPECTED AND APPROVED PRIOR TO PLACING CONCRETE.
9. UNLESS OTHERWISE SHOWN, ASIDE FROM NORMAL ACCESSORIES USED TO HOLD REINFORCING BARS FIRMLY IN POSITION, THE FOLLOWING SHALL BE ADDED:
A) IN SLABS #5 RISER BARS AT 36 INCHES O.C. MAXIMUM TO SUPPORT TOP REINFORCING BARS.
B) IN WALLS WITH 2 CURTAINS #3 U OR Z SHAPE SPACERS AT 6 FEET O.C. EACH WAY.
10. METAL CLIPS OR SUPPORTS SHALL NOT BE PLACED IN CONTACT WITH THE FORMS OR THE SUBGRADE. CONC. BLOCKS (OR DOBBIES) SUPPORTING BARS ON SUBGRADE SHALL BE IN SUFFICIENT NUMBERS TO SUPPORT THE BARS WITHOUT SETTLEMENT, BUT IN NO CASE SHALL SUCH SUPPORT BE CONTINUOUS.
11. DOWELS SHALL BE WIRED OR OTHERWISE HELD IN POSITION. THEY SHALL NOT BE SHOVED INTO FRESHLY PLACED CONCRETE.
12. UNLESS OTHERWISE NOTED ON THE DRAWINGS, SPLICES OF REINFORCEMENT SHALL BE AS SHOWN ON DETAIL 1 ON DWG SD-501.
13. REINFORCING BARS AND ACCESSORIES SHALL NOT BE IN CONTACT WITH ANY PIPE, PIPE FLANGE OR METAL PARTS EMBEDDED IN CONCRETE, A MINIMUM OF 2 INCHES CLEARANCE SHALL BE PROVIDED AT ALL TIMES.
14. ANCHOR BOLT MATERIAL SHALL BE ASTM A307, UNLESS OTHERWISE NOTED.
15. FOR ANCHOR BOLT DETAIL REFER TO TYPICAL DETAIL ON DRAWING SD-50. UNLESS OTHERWISE NOTED ON DESIGN DRAWINGS.
16. EMBEDDED HEADED WELD STUDS SHALL CONFORM TO ASTM A108, UNON.
17. ALL STAIRS AND LADDERS RESTING ON GRADE SHALL BE PROVIDED WITH FOUNDATION PADS PER TYPICAL DETAILS ON DRAWING SD-50, UNLESS OTHERWISE NOTED.
18. UNLESS INDICATED OTHERWISE, CONCRETE COVER OVER REINFORCEMENT SHALL BE AS FOLLOWS:
A. SURFACES NOT EXPOSED DIRECTLY TO THE GROUND, WATER OR WEATHER AFTER FORM REMOVAL:
CONCRETE SLABS IN BUILDINGS 3/4"
CONCRETE SLABS OVER WATER BEARING STRUCTURES 1"
CONCRETE BEAMS, GIRDERS, COLUMNS AND WALLS 1 1/2"
B. FORMED SURFACES EXPOSED DIRECTLY TO THE GROUND, WATER OR WEATHER 2"
C. UNFORMED CONCRETE, PLACED DIRECTLY AGAINST GROUND 3"
19. MAXIMUM SPACING OF CONSTRUCTION JOINTS: 25 FT. HORIZONTAL & 12 FT. VERTICAL.
20. ALL WELDABLE REBARS SHALL BE ASTM A706. WELDING OF A615 IS NOT ALLOWED.

MASONRY

- 1. CONCRETE BLOCK MASONRY SHALL BE MEDIUM WEIGHT HOLLOW UNITS CONFORMING TO ASTM C90, GRADE N-1. SIZE OF UNITS, COLOR AND TEXTURE SHALL BE APPROVED BY THE ARCHITECT.
2. GROUT SHALL CONFORM TO ASTM C476 WITH A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI AT 28 DAYS. AGGREGATE FOR GROUT SHALL CONFORM TO ASTM C404.
3. STEEL REINFORCEMENT FOR REINFORCED MASONRY CONSTRUCTION SHALL CONFORM TO ASTM A615, GRADE 60.
4. SOLID GROUT ALL REINFORCING CELLS OF CONCRETE BLOCK MASONRY UNLESS OTHERWISE NOTED ON DRAWINGS.
5. LAP SPLICE LENGTH SHALL BE AS SHOWN ON SD-501. STAGGER HORIZONTAL LAP SPLICES AT A MINIMUM OF 2'-6" ON CENTER OR 48 BAR DIAMETERS, WHICHEVER IS GREATER.
6. MORTAR SHALL BE TYPE S AND SHALL HAVE MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS OF 1800 PSI. GROUT SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI.
7. UNLESS OTHERWISE SPECIFIED, SPECIAL INSPECTION SHALL BE PROVIDED FOR ALL MASONRY WORK.
8. THE COMBINED MASONRY ASSEMBLAGE COMPRESSIVE STRENGTH AT 28 DAYS SHALL BE A MIN OF f'm = 1500 PSI.
9. REINFORCING STEEL SHALL HAVE MINIMUM 1/2 INCH OR ONE BAR DIAMETER GROUT COVERAGE.
10. NO PIPES OR DUCTS SHALL BE PLACED IN MASONRY WALLS UNLESS SPECIFICALLY NOTED OR DETAILED.
11. REINFORCEMENT SHALL BE SECURED AGAINST DISPLACEMENT PRIOR TO GROUTING USING WIRE POSITIONERS AT INTERVALS NOT EXCEEDING 200 BAR DIAMETER.
12. ALL REINFORCING BAR HOOKS SHOWN ARE STANDARD HOOKS ACCORDING TO ACI 318, UNLESS OTHERWISE NOTED.

ALUMINUM

ALUMINUM CONSTRUCTION SHALL BE IN ACCORDANCE WITH ALUMINUM CONSTRUCTION MANUAL, SPECIFICATION FOR ALUMINUM STRUCTURES, 5TH EDITION. ALL ALUMINUM SHALL BE ALUMINUM ALLOY 6061-T6. ALUMINUM SURFACES SHALL BE PREVENTED FROM COMING IN DIRECT CONTACT WITH CONCRETE OR WITH METALS NOT COMPATIBLE WITH ALUMINUM, USING METHODS DESCRIBED IN THE SPECIFICATIONS.

TESTING HYDRAULIC STRUCTURES

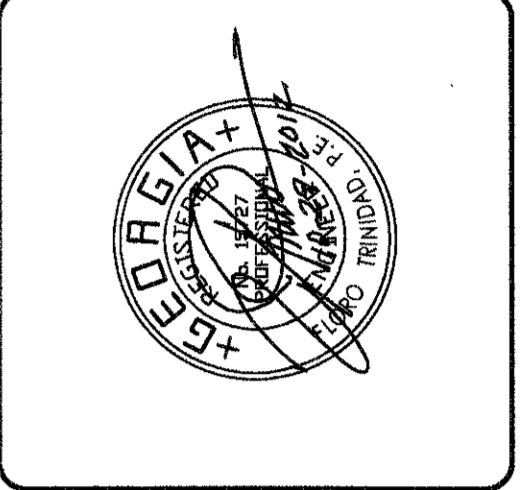
- 1. HYDROSTATIC TESTING AND REQUIRED REPAIR OF HYDRAULIC STRUCTURES SHALL BE COMPLETED PRIOR TO BACKFILLING EXTERIOR WALLS.
2. HYDRAULIC STRUCTURES SHALL NOT BE TESTED PRIOR TO STRUCTURE BEING COMPLETED AND ITS CONCRETE HAVING ATTAINED ITS SPECIFIED COMPRESSIVE STRENGTH.
3. EACH CELL OF A MULTI-CELL STRUCTURE SHALL BE CONSIDERED A SEPARATE STRUCTURE AND BE TESTED INDIVIDUALLY.
4. FOR ADDITIONAL TESTING REQUIREMENTS REFER TO SPECIFICATION 03300, SUBSECTION 3.15. - TESTING FOR WATERTIGHTNESS OF CONCRETE STRUCTURES.

WATERPROOFING

- 1. THE EXTERIOR FACE OF ALL BELOW GRADE WALLS FOR WATER BEARING STRUCTURES INCLUDING EXTERIOR FACE OF WALL FOUNDATIONS SHALL BE WATERPROOFED BY WATERPROOF MEMBRANE PER SPECIFICATIONS. THE WATERPROOF MEMBRANE SHALL BE EXTENDED MINIMUM 6" ABOVE SLAB OR FINISH GRADE LEVEL.

INSPECTION

- CONTINUOUS INSPECTION SHALL BE PERFORMED BY A SPECIAL INSPECTOR OF THE APPROVED INDEPENDENT TESTING LABORATORY AND/OR THE ENGINEER THE FOLLOWING WORK:
1. PLACING OF CONCRETE AND REINFORCING STEEL FOR CONCRETE WITH 28-DAY STRENGTHS IN EXCESS OF 2000 PSI INCLUDING NON-SHRINK GROUT PLACEMENT.
2. PLACING OF MASONRY UNITS AND REINFORCING STEEL FOR CONCRETE BLOCK MASONRY.
3. STRUCTURAL FIELD WELDING.
4. ADHESIVE ANCHORING SYSTEM/ EXPANSION BOLTS
5. SHOTCRETE APPLICATION.
6. FORMWORK
7. HIGH STRENGTH BOLTING
FOR COMPLETE LISTING OF REQUIRED VERIFICATIONS AND INSPECTIONS, AND FREQUENCY OF INSPECTIONS, REFER TO DIVISION 1, SECTION 01400 - SUBSECTION 1.02.A AND 1.07.F.11 A.4 AND 1.07.F.12 FOR STRUCTURAL TESTS AND SPECIAL INSPECTIONS.



PEACHTREE CREEK SOUTH FORK RELIEF STORAGE AND PUMPING STATION
CITY OF ATLANTA
DEPARTMENT OF WATERSHED MANAGEMENT

Table with 4 columns: NO., DATE, REV., DESCRIPTION. Row 1: 0, 10/29/12, REV, 100 PERCENT BID PACKAGE

THIS LINE IS ONE INCH LONG WHEN PLOTTED FULL SCALE.
THIS DRAWING MUST BE USED IN CONJUNCTION WITH THE APPLICABLE OR COVERING TECHNICAL SPECIFICATIONS AND OTHER CONTRACT DOCUMENTS.
PROJECT NO: FC-6260
DATE: OCTOBER 2012
RESP PROF: DESIGNER: CHECKER:
FT FT DSF

SHEET TITLE
STRUCTURAL
00 - GENERAL
GENERAL NOTES

SHEET NO. SD-500 REV. 0